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November 18, 2024

Neel-Schaffer, Inc.
3036 Longfellow Drive
Bay St. Louis, Mississippi 39520

Report No. 240528A

Attention: Alex Davis, P.E.

**Geotechnical Investigation
KHSa Site 04 Corporate Hangar
Stennis International Airport
Kiln, Mississippi**

Gentlemen:

Submitted here are the results of the geotechnical investigation for the above-captioned project. This investigation was authorized through Task Order No. 13 to the Neel-Schaffer, Inc. contract with Burns Cooley Dennis, Inc., and was performed in general accordance with our Proposal No. 24001P-116 dated April 12, 2024. Subsurface soils encountered in the exploratory borings indicate that shallow and grade-supported foundations are appropriate for the planned building providing proper earthwork methods are employed. Recommendations for the KHSa Site 10 Warehouse are presented in our Report No. 240528B.

We appreciate the opportunity to be of service. If you should have any questions concerning this report, please do not hesitate to call us.

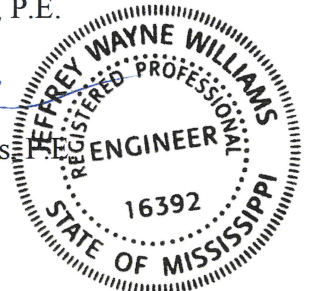
Very truly yours,

BURNS COOLEY DENNIS, INC.

G. Thomas Dunlap

G. Thomas Dunlap, P.E.

Jeffrey W. Williams
Jeffrey W. Williams



JWW/GTD/khb
Copy Submitted: (via e-mail)

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FIGURES

1.0 INTRODUCTION AND PURPOSES

1.1 Project Description

Burns Cooley Dennis, Inc. (BCD) understands that a Corporate Hangar and a Warehouse will be constructed for the Hancock County Port & Harbor Commission (HCPHC) at Stennis International Airport in Hancock County, Mississippi. The Corporate Hangar (which is identified as KHSA Site 04) will be constructed on a 1.4-acre tract located west of Fred and Al Key Road between the Packaging Hangar and Hangar 3 of the Terminal. The results of our investigation for KHSA Site 10 are presented in our Report No. 240528B.

The exact location within the site that the Corporate Hangar will be situated is not yet known. Current plans are for the Corporate Hangar to have a footprint area of about 10,000 to 15,000 sq ft. We understand that the building will be supported on shallow foundations as other buildings at the airport are supported. We assume that the building will be fairly lightly loaded with maximum column loads of 125 kips and maximum wall loads of 4 kips per ft. We also assume that no more than 2 ft of grade increase or decrease will be required to achieve finish grades for the building. The site where the Corporate Hangar will be constructed is covered with grass. One tree was observed at the west end of the site.

1.2 Purposes

The specific purposes of this investigation were:

- 1) to explore subsurface soil conditions by means of borings at locations within the area to be occupied by the building;
- 2) to determine pertinent physical properties of the soils encountered by means of laboratory tests performed on selected representative samples; and
- 3) after analysis of all field and laboratory data, to provide recommendations for site preparation, earthwork construction, and foundation design and construction.

Environmental study, detailed slope and trench stability, retaining wall evaluation, groundwater dewatering, structure foundation design, pavement recommendations, review of plans and specifications, and construction phase services are not a part of the current scope of study.

2.0 FIELD INVESTIGATION

2.1 General

Subsurface soil conditions within the proposed building area were explored by means of four (4) borings. The borings, which were identified as Borings 1 through 4, were made with a tractor-mounted drill rig. Borings 5 through 14 were made for the KHSa Site 10 Warehouse Building and are presented in our Report No. 240258B. Borings 1 through 4 were each made to a depth of 20 ft for the Corporate Hangar. The borings were located and marked in the field by BCD representatives within the middle area of the site where we anticipate that construction is likely. The approximate locations of the borings are illustrated on Figure 1 of this report. Ground surface elevations were not determined as part of our scope.

All soils were classified in general accordance with the Unified Soil Classification System. A legend is shown on Figure 2 which presents a summary of the Unified Soil Classification System and also the symbols and terminology typically utilized on the graphic logs. Graphical logs of the borings indicating the types of soils encountered and stratification at each location explored are shown on Figures 3 through 6. Approximate GPS coordinates for each boring location as determined by our drilling personnel using a hand-held device are shown at the bottom of each graphical boring log within the "Comments" section.

2.2 Drilling Methods and Groundwater Observations

The borings were advanced full depth by dry augering. Observations were made continuously during auger drilling in an attempt to detect groundwater seepage emerging into the open boreholes. Notes pertaining to observed groundwater conditions are indicated in the lower right corner of the graphic logs.

2.3 Sampling Methods

Relatively undisturbed samples of the soils were obtained in each boring by pushing a 3-in. OD Shelby tube sampler approximately 2 ft into the soil. The Shelby tube samples were obtained within the depth intervals illustrated as shaded portions of the "Samples" column of the graphic boring logs. Disturbed samples of soil were also obtained in the borings by driving a standard 2-in. OD split-spoon sampler 18 in. into the soil with a 140-lb hammer falling freely a

distance of 30 in. The depths at which the split-spoon samples were taken are indicated by crossed-slashed symbols under the "Samples" column of the graphic logs. The number of blows required to drive the split-spoon sampler each 6-in. increment (total of 18 in. of penetration) is recorded under the "Field SPT Data" column of the graphic logs. The standard penetration test (SPT) blow count required to penetrate the last foot (sum of the second and third blow count values) is considered the N_{60} value and is plotted as a small circled x in the data section of the graphic logs. The standard penetration test (SPT) blows required to penetrate the last foot (sum of the second and third blow count values) is considered the N_{60} value after applying a correction factor for hammer energy. The Shelby tube and split-spoon samples were obtained at approximate 3 ft to 5-ft intervals of depth in the borings.

Disturbed auger cutting samples were taken near the ground surface in the borings. The depths at which the auger cutting samples were taken are illustrated as small I-shaped symbols under the "Samples" column of the graphic boring logs.

2.4 Field Classifications, Sample Preservation, and Borehole Completion

All soils encountered during drilling were examined and classified in the field by a geotechnical engineering technician. The undisturbed Shelby tube samples were extruded from the sampling tubes in the field. An approximate 6-in. long portion of each Shelby tube sample was sealed with melted paraffin in a cylindrical cardboard container to prevent moisture loss and structural disturbance. An additional portion of each Shelby tube sample, a representative portion of the split-spoon samples, and the auger cutting samples were sealed in jars to provide material for visual examination and testing in the laboratory. Unless other disposition is requested, we routinely discard soil samples after about six months of storage. The boreholes were filled with soil cuttings after completion of drilling and sampling.

3.0 LABORATORY INVESTIGATION

3.1 General

An evaluation of the strength, compressibility, classification, and expansive properties of the subsurface soils encountered in the borings was considered to be of primary importance for this investigation. These properties were evaluated by visual examination and from the results of

compressive strength and routine laboratory index tests described in the following paragraphs. The strength and compressibility characteristics of the soils encountered in the borings were estimated by means of field classifications of consistency, standard penetration tests, and also from results of laboratory compression tests.

3.2 Strength Properties

The strength characteristics of the fine-grained soils encountered in the borings were investigated by means of unconfined compression tests performed on selected Shelby tube samples. The results of unconfined compression tests are plotted as small open circles in the data section of the graphic logs. The water content and dry density were also determined for each compression test specimen. The actual consistency assigned on the boring logs also considers other data including physical examination of the sample. The water contents are plotted as small shaded circles in the data section of the logs. The dry densities are tabulated to the nearest lb per cu ft under the “Dry Density” column of the logs.

3.3 Classification Tests

The classification and expansive properties of the soils encountered were investigated by means of visual examination and Atterberg liquid and plastic limit tests. The numerical difference between the liquid and plastic limits is defined as the plasticity index (PI). The magnitude of the plasticity index and the proximity of the natural water content to the plastic limit are indicators of the potential for a clay to shrink or swell upon changes in moisture content or to consolidate under loading. The magnitude of the plastic limit is an indicator of the optimum moisture content for compaction, and the proximity of the natural water content to the plastic limit is an indicator of the strength. The results of the liquid and plastic limit tests are plotted as small crosses interconnected by dashed lines in the data section of the logs.

To assist in classifying soils which include sand, tests were performed to determine the percent fines passing the No. 200 sieve. The percentage of fines resulting from each test is tabulated at the appropriate depth under the far-right column of the graphic logs.

3.4 Water Content Tests

Water content tests were performed on samples to corroborate field classifications and to extend the usefulness of the plasticity data. The results of the water content tests are plotted as small shaded circles in the data section of the graphic boring logs. The water content data have been interconnected on the logs to illustrate a continuous profile with depth.

4.0 GENERAL SOIL CONDITIONS

4.1 General

A general description of subsurface soil and groundwater conditions encountered in the borings made for this investigation is provided in the following subsections. The graphical logs of the borings shown on Figures 3 through 6 should be referred to for the specific soil conditions encountered at each location explored. Stick logs of the borings are shown in profile on Figure 7 to aid in visualizing subsurface soil conditions. Tabulated adjacent to the stick logs are water contents, Atterberg liquid and plastic limits, dry densities, SPT blow counts, and cohesions.

4.2 Soil Stratification

The generalized soil conditions encountered in our borings are as follows:

STRATUM	TYPICAL BOTTOM DEPTH	MATERIAL DESCRIPTION	DENSITY/ CONSISTENCY
1	2.5 ft	Sandy Silts (ML)	Loose to Medium Dense
2a	4 ft	Silty Clays (CL) and Sandy Clays (CL)	Soft to Medium Stiff
2b	8 ft	Sandy Clays (CL)	Stiff
3	17 ft	Clays (CH)	Medium Stiff to Stiff
4	20 ft (Termination Depth)	Silty Sands (SM) and Clayey Sands (SC)	Loose to Medium Dense

The loose to medium dense silts (ML) and sands (SM and SC) are considered to have low-moderate to moderate strength and moderate to moderate-high potential for elastic settlement. The soft to medium stiff clays (CL and CH) are considered to have low to low-moderate strength and moderate-high to high compressibility. The stiff clays (CL and CH) are considered to have moderate strength and compressibility.

The silts (ML), silty clays (CL), sandy clays (CL), and sands (SM and SC) are considered to have low or no shrink/swell potential. The clays (CH) are considered to be expansive with high shrink/swell potential.

4.3 Groundwater Observations

As indicated previously, observations were made continuously during auger drilling to detect groundwater seepage entering the open boreholes. Free water was not encountered in the borings during auger drilling. It should be recognized that the groundwater levels at this site are influenced by rainfall, water levels in the nearby ditches, creeks, ponds or other bodies of water, local drainage conditions, and other environmental factors. Groundwater conditions at the site can also be influenced by man-made changes. Surficial soils can become saturated and weak to relatively shallow depths during periods of prolonged and heavy rainfall. Surface water may become perched within the surficial sandy silts (ML) upon the underlying less permeable silty clays (CL) and sandy clays (CL).

5.0 DISCUSSION

Generally, the surficial sandy silts (ML) were relatively strong at the time of our field investigation. However, the sandy silts (ML) encountered in Boring 3 from the ground surface to a depth of 4 ft were relatively weak and unstable. Additionally, the soft to medium stiff silty clays (CL) and sandy clays (CL) encountered beneath the surficial silts (ML) to a depth of about 4 ft in Borings 1, 2, and 4 are relatively weak and unstable. The presence of these unstable soils will have to be addressed through removal and replacement with select fill materials during site preparation.

Expansive clays (CH) such as those encountered below a depth of 8 ft in each boring will tend to shrink during dry conditions and swell during wet conditions. We do not consider that the presence of the expansive clays (CH) will adversely affect foundation performance due to their depth of confinement below the ground surface.

A foundation for the Site 04 Corporate Hangar should be utilized which will accommodate the anticipated fill and structural loads and also minimize future differential vertical movements resulting from settlement due to soil consolidation/compression under fill and structural loads. For

the soil conditions revealed by the borings, it is our opinion that a shallow foundations system could be used for support of the structure provided: 1) column loads are less than 125 kips and wall loads do not exceed 4 kips per ft; 2) site grades will not be increased or decreased by more than 2 or 3 ft; and 3) our recommendations for site preparation and earthwork construction are implemented.

Details of our recommendations for site preparation, earthwork construction, and for the design and construction of the recommended foundation systems are included in the following subsections.

6.0 RECOMMENDATIONS

6.1 Site Preparation and Earthwork Construction

6.1.1 General. As an initial step of site preparation within the planned Site 04 Corporate Hangar construction area, stripping should be performed throughout the construction area to remove organic-laden surficial soils, vegetation, debris, brush and roots and the stability of the exposed soils should be evaluated by proofrolling the building area where practical. As previously mentioned, the loose sandy silts (ML) and soft to medium stiff clays (CL) encountered directly beneath the ground surface were found to be relatively weak to a depth of about 4 ft at the time of our investigation. Overexcavation to expose stable soils and replacement of unstable soils with compacted select fill material will likely be necessary if the soil conditions do not improve through drying prior to construction. In general, excavation should extend to a sufficient depth to expose soils that will support compaction with stability.

The actual vertical and lateral extent of excavation required to remove weak or unstable soils must be determined in the field during earthwork construction. Excavation of any weak soils should extend laterally not less than 5 ft beyond the edges of the building. In order to minimize the amount of excavation, we recommend that a representative of Burns Cooley Dennis, Inc. be present to observe excavation operations and assist in evaluating the depth and lateral extent of excavation required.

Excavations made to remove unstable soils and any tree root systems should be backfilled with select fill materials in accordance with recommendations provided in a subsequent paragraph. Excavated soils that meet the requirements of select fill may be stockpiled for later use as backfill.

The actual moisture content conditions at the time of construction will be influenced by the rainfall preceding and during construction and also by the drying conditions during construction. Therefore, we recommend that the work be performed during a dry season of the year if the schedule permits.

6.1.2 Groundwater Seepage Control. Free water was not encountered in our borings. Depending on the season when earthwork is performed, groundwater may be encountered during excavation. It should be noted that surface water is likely to become perched within the silts (ML) upon the underlying less permeable clays (CL) encountered at depths of 2 ft to 4 ft below grade. The means and methods for intercepting, collecting and removing groundwater entering excavations should be the sole responsibility of the earthwork contractor.

6.1.3 Subgrade Preparation. Prior to the placement of any fill materials, the soils exposed after stripping and excavation should be scarified to a minimum depth of 6 in. and compacted to not less than **95 percent** of standard Proctor maximum dry density (ASTM D 698) with stability present. Alternatively, the exposed soils could be proofrolled with loaded dump trucks to demonstrate stability. Stability is defined as the absence of significant pumping, rutting or yielding of soils during compaction or proofrolling.

If stability is not evident in some areas, either additional excavation or treatment of the in-situ soils with an admixture, or a combination of these approaches, might be required to achieve stable conditions.

The effort required to mitigate unstable soils will be influenced by the season of the year when earthwork is performed. The subsurface soils would likely be drier during the hot late summer and could weaken during heavy rainfall events. We recommend that earthwork be performed during a dry summer or fall season, if the schedule permits. It should be recognized that soils which are demonstrated to be adequately stable during stripping and excavation can become unstable if they are disturbed by construction traffic or if they are exposed to rainfall prior to filling.

The construction techniques, types of equipment utilized and site drainage provided during construction will have a great effect on the performance of the exposed soils. The routing of heavy, rubber-tired equipment should be controlled to minimize, as much as possible, traffic in the construction areas. All traffic should be discouraged during periods of inclement weather. If

pumping is initiated in the soils, as a construction expedient the pumping can be counteracted by treating these materials with lime. It is estimated that about 4 to 6 percent lime by dry weight of soil could be required.

6.1.4 Delays. If there is some delay between completion of earthwork operations and placement of the new fill, deterioration of subgrade conditions may occur. Thus, we recommend that the density and stability of the subgrade soils be confirmed or re-established immediately prior to construction of structure footings and slab.

6.2 Fill Materials and Placement

6.2.1 Select Fill. After stripping, undercutting, and visual evaluation or compaction/densification have been performed as recommended in the preceding paragraphs, fill materials can be placed to achieve planned grades. Imported fill materials should consist of select nonorganic and debris-free silty clays (CL) or sandy clays (CL) having a liquid limit less than 45 and a plasticity index (PI) within the range of 10 to 24, or clayey sands (SC) with a PI in the range of 7 to 15 and a liquid limit less than 35. Existing silty clays (CL) and sandy clays (CL) excavated during site preparation can be used as select fill, provided they are free of organic matter and other deleterious materials, they meet the classification requirements for imported select fill, and they are not too wet or dry and can be processed to achieve a moisture content within 3 percentage points of the optimum water content. The existing silts (ML) encountered directly beneath the ground surface will be especially susceptible to instability when wet and should only be reused as select fill if the contractor understands that the soils will be very difficult to compact when wet. Soils containing organic matter and other deleterious materials should be placed outside the limits of the proposed building area.

6.2.2 Fill Placement. The fill materials should be compacted from lifts not exceeding 9 in. in loose thickness to not less than **95 percent** of standard Proctor (ASTM D 698) maximum dry density with stability present. Fill materials should be placed in horizontal lifts at moisture contents within 3 percentage points of the optimum water content. Stability must be evident during compaction of each lift before any subsequent lifts of fill material are added. All backfill over utilities, storm drains, etc., should be placed in accordance with the preceding recommendations,

except the lift thickness should be reduced to about 4 in. where hand-operated compaction equipment is used.

Finished site grades should be sloped to promote quick runoff of storm water and provide positive drainage away from the building on all sides. Fill materials should extend laterally not less than 5 ft beyond the perimeter of the building and then slope down to natural ground at an inclination not steeper than 3H:1V. Finished site grades should be sloped to promote quick runoff of storm water and provide positive drainage away from the building.

Increased moisture contents resulting from rainfall can substantially reduce the strength of the on-site and proposed fill soils. We recommend that proper surface drainage be provided throughout the site to prevent surface water from percolating into the subsurface soils.

6.2.3 Laboratory and Field Testing. Laboratory classification tests, including Atterberg limit determinations and grain-size analyses, should be performed on the fill soils initially and routinely during earthwork operations to check for compliance with the recommendations provided herein. Field moisture/density tests should be performed frequently in the compacted on-site soils and in each compacted lift of fill material to assist in evaluating whether the recommended moisture contents and dry densities are being achieved. As a guide for earthwork construction, we suggest a minimum of one test per lift per 2,500 sq ft of surface area within the building area. We recommend a minimum of two tests per lift.

6.3 Foundation Design

6.3.1 General. In conjunction with the recommended earthwork construction, it is our opinion that the planned building can be supported on a shallow foundation system with the floor slab cast on grade provided the structure is not heavily loaded. The allowable capacities provided are for column loads no greater than 125 kips and continuous loads no greater than 4 kips per ft.

6.3.2 Foundation and Slab Design. The foundations for the Site 04 Corporate Hangar building can consist of strip footings supporting wall loads, spot footings supporting column loads, and a separate slab on-grade. Spot footings and exterior strip footings should be brought to bear at a depth of not less than 24 in. below finished outside grades adjacent to the building or the bottom of the floor slab, whichever is lower. Interior grade beams or strip footings should extend to a

depth of not less than 18 in. below the bottom of the floor slab. The grade beams or strip footings should be proportioned for critical combinations of dead, live, and wind loads using a net allowable soil bearing pressure of 2,000 lbs per sq ft. We recommend a minimum base width of 18 in. for the grade beams or strip footings. Square or spot footings with widths of 24 in. or greater should be designed assuming a net allowable soil bearing pressure of 2,500 lbs per sq ft when total dead plus live loads are applied. These net allowable bearing pressures include an appropriate factor of safety with respect to soil shear strength.

Spot footings can be connected with grade beams if needed to better accommodate structural loadings. Footings can be designed for lateral loading considering an allowable passive equivalent fluid unit weight of 200 lbs per cu ft per ft and a sliding resistance coefficient against the base of 0.4, where lateral loads against the structure are not accommodated in uplift and compression.

The uplift capacity of each footing should be computed as the weight of the concrete plus the weight of the backfill immediately above the footing base. The weight of the backfill soils should be computed using a unit weight of 120 lbs per cu ft. The unit weight of the concrete should be taken as 150 lbs per cu ft. The size of the base or the bearing depth of the footing should be increased to provide an uplift capacity that will result in an adequate safety factor against uplift loads as determined by the designer.

The building floor slab can be supported by properly compacted subgrade soils or by select fill materials placed and compacted in accordance with the recommendations given previously in this report. The floor slab should be adequately reinforced for anticipated loading conditions and deflections and to minimize slab cracking. Stiffening ribs or grade beams cast monolithically with the slabs could be utilized to provide rigidity.

6.3.3 Stiffened Slab-on-Grade Foundation. The new Site 04 Corporate Hangar could alternatively be supported by a foundation system consisting of a slab-on-grade stiffened with perimeter grade beams, or turned-down edges, and interior grade or tie beams. Grade beams should be utilized to support all exterior walls and all interior load-bearing and partition walls, or otherwise they should be spaced in a grid pattern on not greater than about 15-ft to 20-ft centers in each direction, or as otherwise directed by the project structural engineer. Any columns should be supported by widened portions of the grade beams. We recommend that grade beams or turned-

down edges around the perimeter of the building be brought to bear at a depth not less than 24 in. below finished outside grade. Interior tie or grade beams should be brought to bear at a depth not less than 18 in. below the bottom of the slab. We recommend that grade beams be proportioned for critical combinations of dead, live, and wind loads utilizing a net allowable soil bearing pressure of 2,000 lbs per sq ft. A net allowable soil bearing pressure of 2,500 lbs per sq ft should be utilized to dimension widened portions of grade beams used to support column loads. We recommend a minimum base width of 12 in. for the grade beams. The grade beams should be reinforced for both positive and negative bending. The floor slab should be reinforced for anticipated loading conditions and deflections and to minimize slab cracking. We recommend that the slab be reinforced with a grid of relatively closely spaced reinforcing bars in lieu of welded wire fabric.

6.3.4 Potential Settlement. We estimate that total settlements under compressive structural loads will be on the order of 1 in. Differential settlements between adjacent footings may be on the order of 3/4 in. Differential settlements are not expected to be more than 3/4 in. over a horizontal distance of 25 ft or 30 ft.

6.4 Foundation Construction

All foundation excavations should be carefully inspected by a knowledgeable soil technician or engineer to verify bearing on generally strong soils. Foundation elements should be promptly poured after completion of the excavations. Drainage should be maintained away from the foundation excavations during construction. Ponded water in the foundation excavations can weaken the exposed soils. Foundation soils exposed in the bottom of the foundation excavations should be inspected prior to concrete placement. If these materials are found to be weak or loose, overexcavation and backfilling will be required to provide strong foundation soils immediately beneath the grade beams and footings.

It should be noted that foundation excavations made through on-site low plasticity silty soils could slough. The contractor should be made aware of this issue. Low plasticity silts will also be susceptible to erosion and surface protection will need to be provided. The best method for protecting the surface is plating the low plasticity soils with about 12 in. to 18 in. of silty clays (CL)

or sandy clays (CL). This plating material along with some topsoil would also help facilitate growth of grass.

6.5 Other Design Considerations

The site for the proposed Site 04 Corporate Hangar at the Stennis International Airport in Kiln in Hancock County, Mississippi lies within a relatively low seismic activity region according to the seismic zone mapping referenced in the International Building Code. Given the site soil profile as revealed by the borings and anticipated for the area based on our experience, a site class D could be used in a seismic load evaluation.

7.0 REPORT LIMITATIONS

The analyses, conclusions, and recommendations included in this report are based on conditions which existed at the time of our field investigation and further on the assumption that the exploratory borings are representative of subsurface conditions throughout the areas planned for construction. It should be noted that actual subsurface conditions between borings might differ from those encountered at the boring locations. Additional borings may be required if the location of the building varies significantly from where the borings were made. If subsurface conditions are encountered during construction that vary from those discussed in this report, Burns Cooley Dennis, Inc. should be notified immediately in order that we may evaluate the effects, if any, on design and construction.

Burns Cooley Dennis, Inc. should be retained for a general review of project plans and specifications. It is advised that we be retained to observe earthwork and foundation construction for the project in order to help confirm that our recommendations are valid or to modify them accordingly. Burns Cooley Dennis, Inc. cannot assume responsibility or liability for the adequacy of recommendations if we do not observe construction.

This report has been prepared for the exclusive use of Neel-Schaffer, Inc. for specific application to the geotechnical-related aspects of design and construction of the KHSa Site 04 Corporate Hangar at Stennis International Airport in Kiln in Hancock County, Mississippi. The only warranty made by us in connection with the services provided is that we have used that degree of care and skill ordinarily exercised under similar conditions by reputable members of our

profession practicing in the same or similar locality. No other warranty, express or implied, is made or intended.

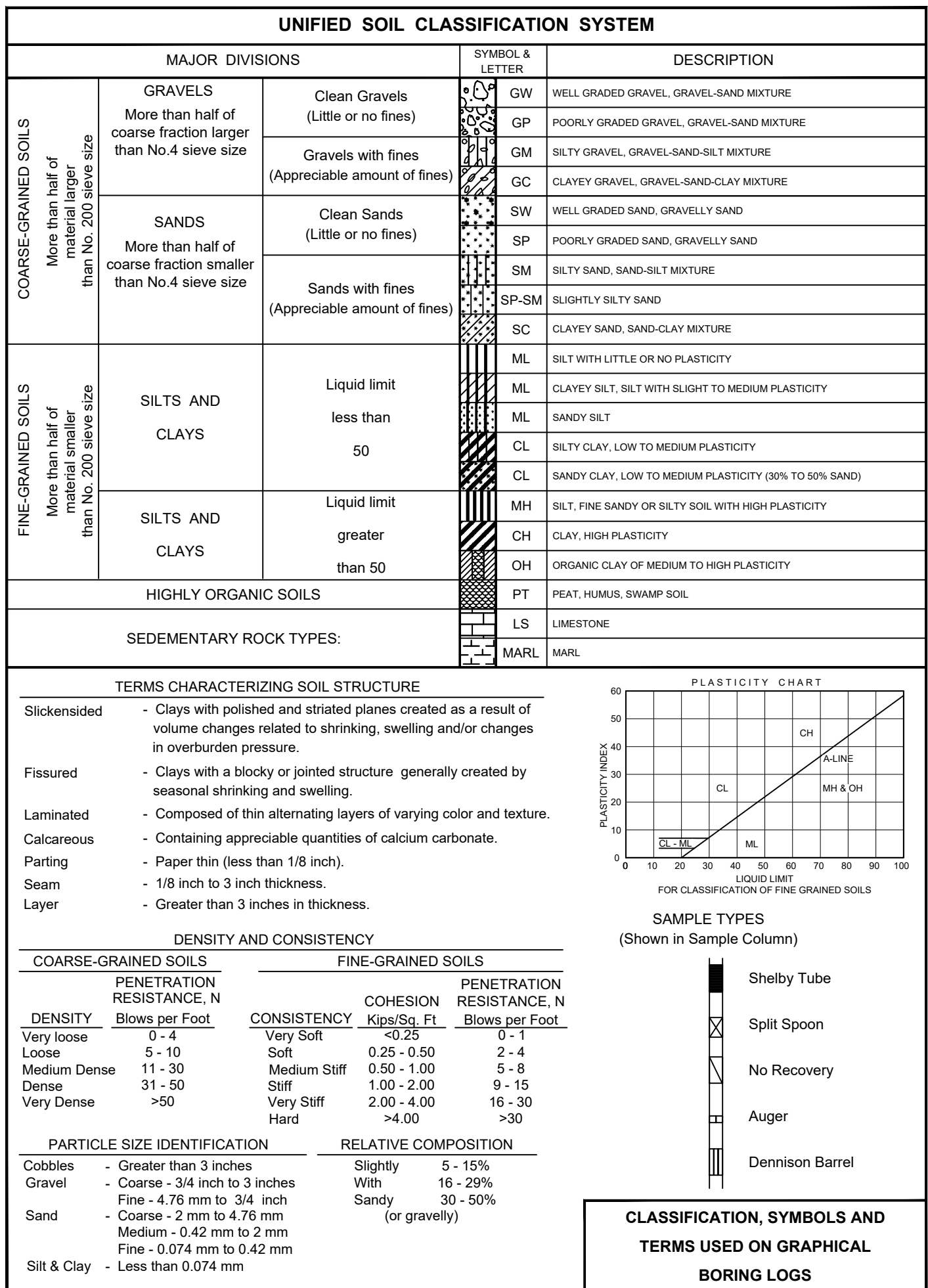
FIGURES



Approximate Boring Locations

KHSA SITE 04 CORPORATE HANGAR
STENNIS INTERNATIONAL AIRPORT
KILN, MISSISSIPPI

BURNS COOLEY DENNIS, INC.
551 SUNNYBROOK ROAD
RIDGELAND, MISSISSIPPI 39157



LOG OF BORING NO. 1

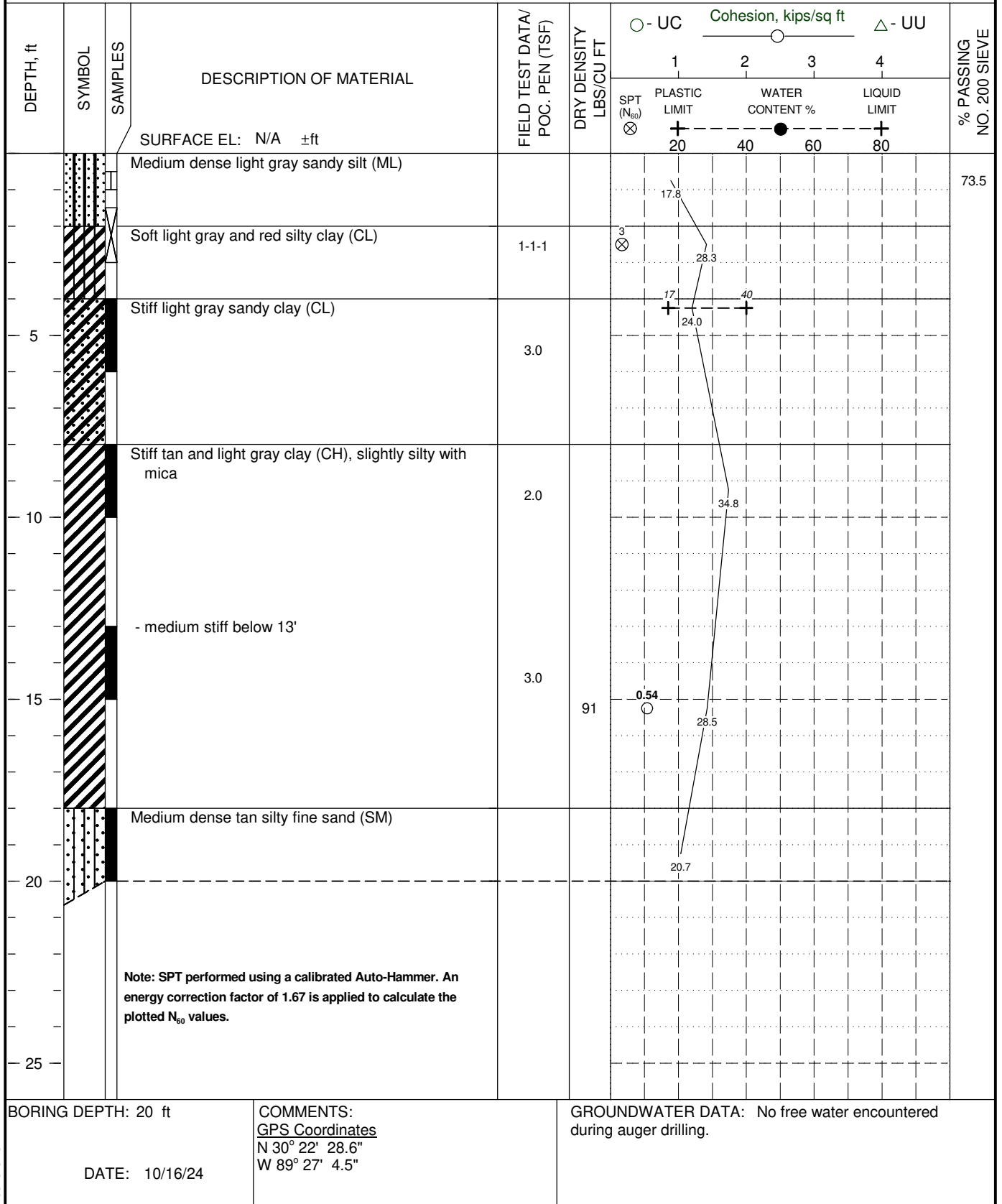
KHSa SITE 04 CORPORATE HANGAR

STENNIS INTERNATIONAL AIRPORT

KILN, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1A



240528.GPJ

LOG OF BORING NO. 2

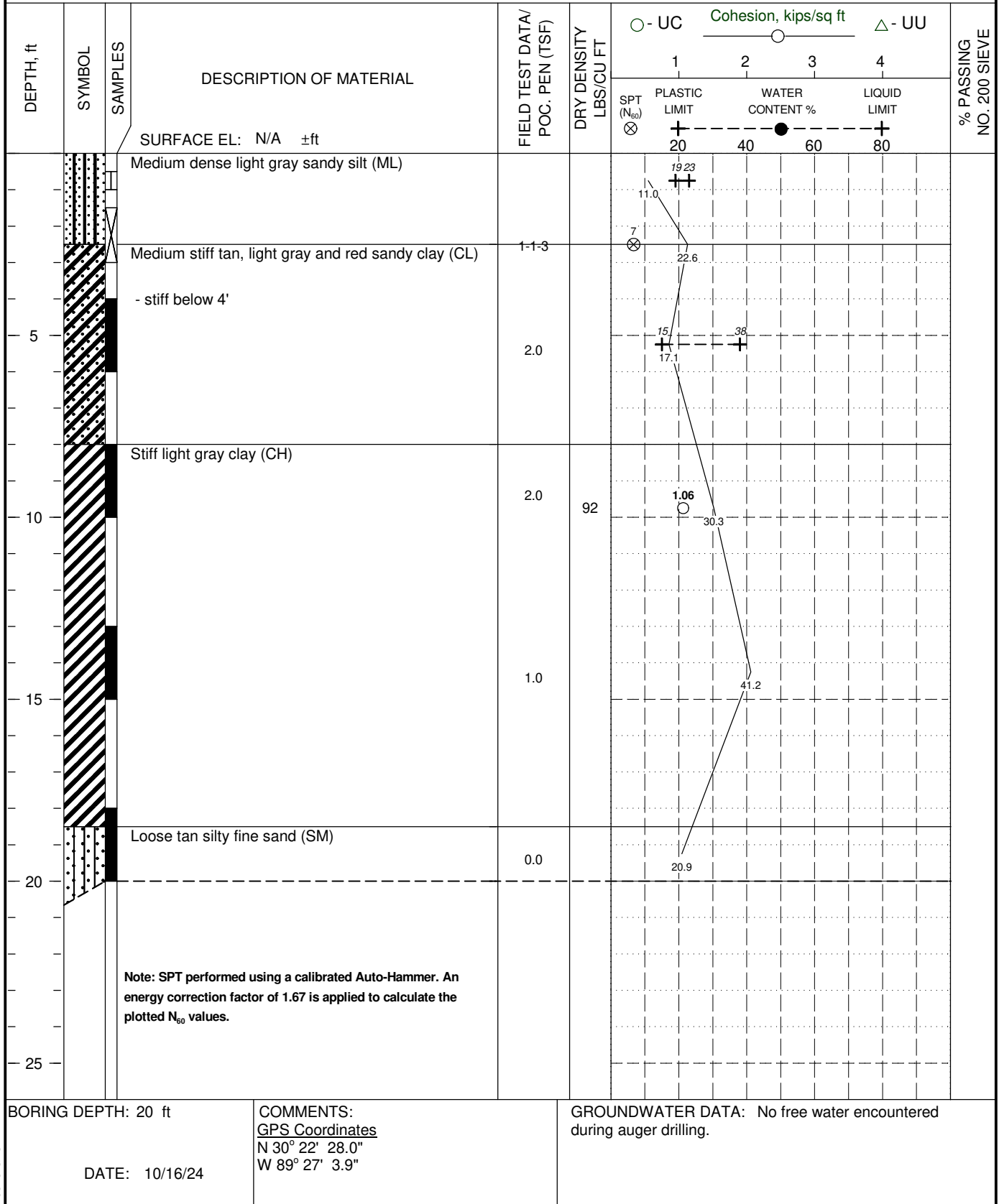
KHSA SITE 04 CORPORATE HANGAR

STENNIS INTERNATIONAL AIRPORT

KILN, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1A



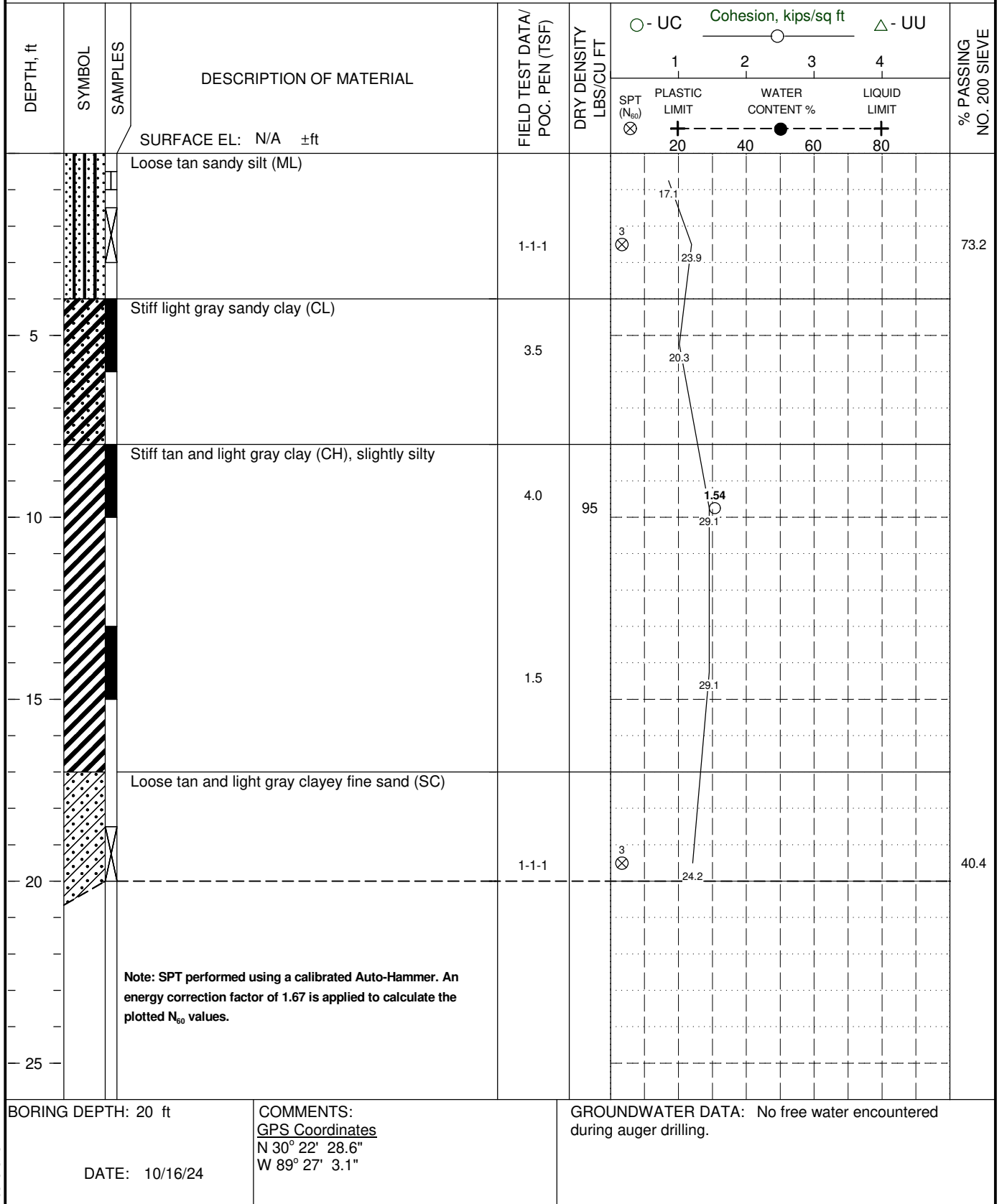
240528.GPJ

LOG OF BORING NO. 3

KHSA SITE 04 CORPORATE HANGAR
STENNIS INTERNATIONAL AIRPORT
KILN, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1A



240528.GPJ

LOG OF BORING NO. 4

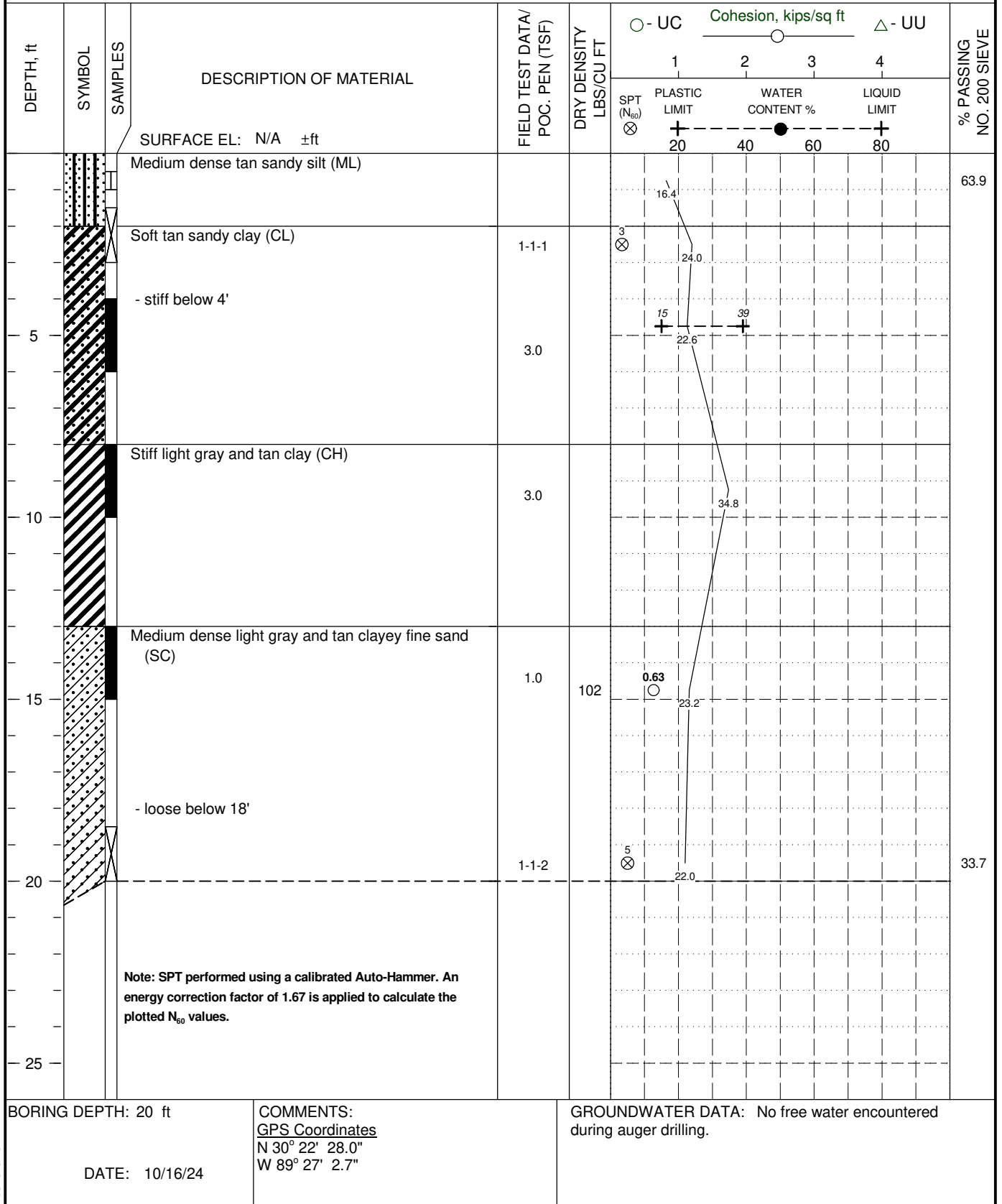
KHSA SITE 04 CORPORATE HANGAR

STENNIS INTERNATIONAL AIRPORT

KILN, MISSISSIPPI

TYPE: 4" Continuous-flight auger

LOCATION: See Figure 1A



240528.GPJ

